World Housing Encyclopedia

an Encyclopedia of Housing Construction in Seismically Active Areas of the World



an initiative of Earthquake Engineering Research Institute (EERI) and International Association for Earthquake Engineering (IAEE)

HOUSING REPORT Wood panel wall buildings (typical seria 181-115-77 cm of "Giprolesprom")

| Report # | 57 |
|------------------|--|
| Report Date | 05-06-2002 |
| Country | RUSSIAN FEDERATION |
| Housing Type | Timber Building |
| Housing Sub-Type | Timber Building : Wood panel walls |
| Author(s) | Mark Klyachko, Andrey Benin, Janna Bogdanova |
| Reviewer(s) | Svetlana Uranova |

Important

This encyclopedia contains information contributed by various earthquake engineering professionals around the world. All opinions, findings, conclusions & recommendations expressed herein are those of the various participants, and do not necessarily reflect the views of the Earthquake Engineering Research Institute, the International Association for Earthquake Engineering, the Engineering Information Foundation, John A. Martin & Associates, Inc. or the participants' organizations.

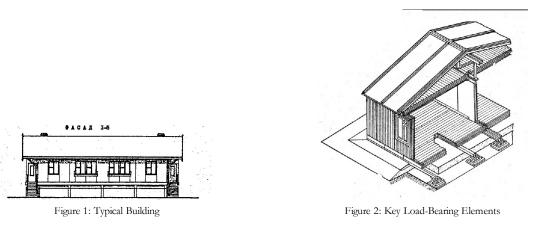
Summary

This is a rural housing construction practice widespread in the forested areas of Russia. Buildings of this type are common in seismically prone areas of Russia (Far East, Siberia, Baikal Lake Region). The load-bearing structure is made of wood panel walls. Buildings have timber roof and fieldstone or concrete strip foundations. Typical seria 181-115-77 cm of 'Giprolesprom' for seismic regions is an example of this building type. Seismic resistance is relatively high, provided that the quality of materials and the construction are satisfactory.

1. General Information

Buildings of this construction type can be found in several seismically prone areas of Russia (induding Far East, Siberia, Baikal Lake Region) where this construction accounts for 5 to 100% of the housing stock. This type of housing construction is commonly found in rural areas. This construction type has been in practice for less than 75 years.

Currently, this type of construction is being built. The Soviet Union construction practice followed in the past 50 years.



2. Architectural Aspects

2.1 Siting

These buildings are typically found in flat terrain. They do not share common walls with adjacent buildings. It can be more than 10 meters also When separated from adjacent buildings, the typical distance from a neighboring building is 10 meters.

2.2 Building Configuration

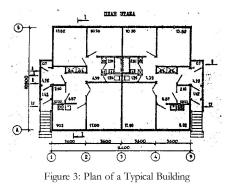
All buildings of this type are rectangular in plan. Windows: 10-15%; Doors: 5-8%.

2.3 Functional Planning

The main function of this building typology is multi-family housing. In a typical building of this type, there are no elevators and 1-2 fire-protected exit staircases. Each housing unit has its own entrance.

2.4 Modification to Building

Modifications in buildings of this type are not common.



3. Structural Details

3.1 Structural System

| Material | Type of Load-Bearing Struc | cture # | Subtypes | Most appropriate type |
|----------|----------------------------|---------|--|-----------------------|
| | Stone Masonry Walls | 1 | Rubble stone (field stone) in mud/lime mortar or without mortar (usually with timber roof) | |
| | w ans | 2 | Dressed stone masonry (in lime/cement mortar) | |
| | | 3 | Mud walls | |
| | Adobe/ Earthen Walls | 4 | Mud walls with horizontal wood elements | |
| | | 5 | Adobe block walls | |
| | | 6 | Rammed earth/Pise construction | |
| | | 7 | Brick masonry in mud/lime mortar | |
| | Unreinforced masonry | 8 | Brick masonry in mud/lime mortar with vertical posts | |
| Masonry | w alls | 9 | Brick masonry in lime/cement mortar | |
| | | 10 | Concrete block masonry in cement mortar | |
| | | 11 | Clay brick/tile masonry, with wooden posts and beams | |
| | Confined masonry | 12 | Clay brick masonry, with concrete posts/tie columns and beams | |
| | | 13 | Concrete blocks, tie columns and beams | |
| | | 14 | Stone masonry in cement mortar | |
| | Reinforced masonry | 15 | Clay brick masonry in cement mortar | |
| | | 10 | Concrete block masonry in cement mortar | |
| | | 17 | 7 Flat slab structure | |
| | | 18 | Designed for gravity loads only, with URM infill walls | |
| | Moment resisting frame | 19 | Designed for seismic effects, with URM infill walls | |
| | | 20 | Designed for seismic effects, with structural infill walls | |
| | | 21 | Dual system – Frame with shear wall | |
| | rete Structural wall | 22 | Moment frame with in-situ shear walls | |

| | | 23 | Moment frame with precast shear walls | |
|--------|------------------------------|----|---|--|
| | | | Moment frame | |
| | | 25 | Prestressed moment frame with shear walls | |
| | Precast concrete | 26 | Large panel precast walls | |
| | | 27 | Shear wall structure with walls cast-in-situ | |
| | | 28 | Shear wall structure with precast wall panel structure | |
| | | 29 | With brick masonry partitions | |
| | Moment-resisting frame | 30 | With cast in-situ concrete walls | |
| | | 31 | With lightweight partitions | |
| Steel | Braced frame | 32 | Concentric connections in all panels | |
| | | 33 | Eccentric connections in a few panels | |
| | Structural wall | 34 | Bolted plate | |
| | | 35 | Welded plate | |
| | | 36 | Thatch | |
| | | 37 | Walls with bamboo/reed mesh and post (Wattle and Daub) | |
| | | 38 | Masonry with horizontal beams/planks at intermediate levels | |
| Timber | Load-bearing timber frame | 39 | Post and beam frame (no special connections) | |
| | | 40 | Wood frame (with special connections) | |
| | | 41 | Stud-wall frame with plywood/gypsum board sheathing | |
| | | 42 | Wooden panel walls | |
| | | 43 | Building protected with base-isolation systems | |
| Other | Seismic protection systems | 44 | Building protected with seismic dampers | |
| | Hybrid systems | 45 | other (described below) | |

3.2 Gravity Load-Resisting System

The vertical load-resisting system is others (described below). Wood panel walls.

3.3 Lateral Load-Resisting System

The lateral load-resisting system is others (described below). Same as vertical load-resisting system.

3.4 Building Dimensions

The typical plan dimensions of these buildings are: lengths between 14.4 and 14.4 meters, and widths between 10.8 and 10.8 meters. The building is 1 storey high. The typical span of the roofing/flooring system is 3.6 meters. The typical storey height in such buildings is 2.7 meters. The typical structural wall density is none. 8-12%.

3.5 Floor and Roof System

| Material | Description of floor/roof system | Most appropriate floor Most appropriate roof |
|----------|----------------------------------|--|
| | | |

| Masonry | Vaulted | |
|---------------------|---|--|
| Masonny | Composite system of concrete joists and masonry panels | |
| | Solid slabs (cast-in-place) | |
| | Waffle slabs (cast-in-place) | |
| | Flat slabs (cast-in-place) | |
| | Precast joist system | |
| Structural concrete | Hollow core slab (precast) | |
| | Solid slabs (precast) | |
| | Beams and planks (precast) with concrete topping (cast-in-situ) | |
| | Slabs (post-tensioned) | |
| Steel | Composite steel deck with concrete slab (cast-in-situ) | |
| | Rammed earth with ballast and concrete or plaster finishing | |
| | Wood planks or beams with ballast and concrete or plaster finishing | |
| | Thatched roof supported on wood purlins | |
| | Wood shingle roof | |
| Timber | Wood planks or beams that support clay tiles | |
| | Wood planks or beams supporting natural stones slates | |
| | Wood planks or beams that support slate, metal, asbestos-cement or plastic corrugated sheets or tiles | |
| | Wood plank, plywood or manufactured wood panels on joists supported by beams or walls | |
| Other | Described below | |

3.6 Foundation

| Туре | Description | Most appropriate type |
|--------------------|--|-----------------------|
| | Wall or column embedded in soil, without footing | |
| | Rubble stone, fieldstone isolated footing | |
| | Rubble stone, fieldstone strip footing | |
| Shallow foundation | Reinforced-concrete isolated footing | |
| | Reinforced-concrete strip footing | |
| | Mat foundation | |
| | No foundation | |
| | Reinforced-concrete bearing piles | |
| | Reinforced-concrete skin friction piles | |
| Deep foundation | Steel bearing piles | |
| Deep toundation | Steel skin friction piles | |
| | Wood piles | |
| | Cast-in-place concrete piers | |
| | Caissons | |
| Other | Described below | |

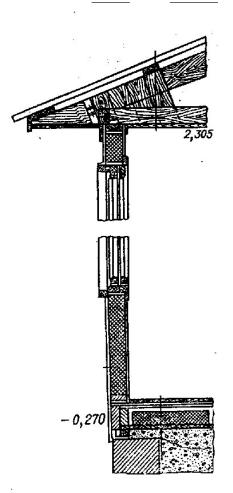


Figure 4: Critical Structural Details - Vertical Sections Through the Wall

4. Socio-Economic Aspects

4.1 Number of Housing Units and Inhabitants

Each building typically has 1 housing unit(s). 2 units in each building. The number of inhabitants in a building during the day or business hours is less than 5. The number of inhabitants during the evening and night is less than 5.

4.2 Patterns of Occupancy

One family per unit (apartment).

4.3 Economic Level of Inhabitants

| Income class | Most appropriate type | | |
|--------------------------------------|-----------------------|--|--|
| a) very low-income class (very poor) | | | |
| b) low-income class (poor) | | | |
| c) middle-income class | | | |
| d) high-income class (rich) | | | |

| Ratio of housing unit price to annual income | Most appropriate type |
|--|-----------------------|
| 5:1 or worse | |
| 4:1 | |
| 3:1 | |
| 1:1 or better | |

| What is a typical source of financing for buildings of this type? | Most appropriate type | | |
|---|-----------------------|--|--|
| Owner financed | | | |
| Personal savings | | | |
| Informal network: friends and relatives | | | |
| Small lending institutions / micro- finance institutions | | | |
| Commercial banks/mortgages | | | |
| Employers | | | |
| Investment pools | | | |
| Government-owned housing | | | |
| Combination (explain below) | | | |
| other (explain below) | | | |

In each housing unit, there are 1 bathroom(s) without toilet(s), no toilet(s) only and no bathroom(s) induding toilet(s).

7

ПГ

Usually one bathroom per one family (unit), i.e.2 bathrooms per building. .

4.4 Ownership

The type of ownership or occupancy is outright ownership and long-term lease.

| Type of ownership or occupancy? | Most appropriate type |
|---|-----------------------|
| Renting | |
| outright ownership | |
| Ownership with debt (mortgage or other) | |
| Individual ownership | |
| Ownership by a group or pool of persons | |
| Long-term lease | |
| other (explain below) | |

Own outright (applies to a housing unit), Long-term lease (typical).

5. Seismic Vulnerability

5.1 Structural and Architectural Features

| Statement | Most a | pprop | iate type |
|--|---|---|--|
| | Yes | No | N/A |
| The structure contains a complete load path for seismic force effects from any horizontal direction that serves to transfer inertial forces from the building to the foundation. | | | |
| The building is regular with regards to both the plan and the elevation. | | | |
| The roof diaphragm is considered to be rigid and it is expected that the roof structure will maintain its integrity, i.e. shape and form, during an earthquake of intensity expected in this area. | | | |
| The floor diaphragm(s) are considered to be rigid and it is expected that the floor structure(s) will maintain its integrity during an earthquake of intensity expected in this area. | | | |
| There is no evidence of excessive foundation movement (e.g. settlement) that would affect the integrity or performance of the structure in an earthquake. | | | |
| The number of lines of walls or frames in each principal direction is greater than or equal to 2. | | | |
| Height-to-thickness ratio of the shear walls at each floor level is: Less than 25 (concrete walls); Less than 30 (reinforced masonry walls); Less than 13 (unreinforced masonry walls); | | | |
| Vertical load-bearing elements (columns, walls) are attached to the foundations; concrete columns and walls are doweled into the foundation. | | | |
| Exterior walls are anchored for out-of-plane seismic effects at each diaphragm level with metal anchors or straps | | | |
| The total width of door and window openings in a wall is: For brick masonry construction in cement mortar : less than ½ of the distance between the adjacent cross walls; For adobe masonry, stone masonry and brick masonry in mud mortar: less than 1/3 of the distance between the adjacent cross walls; For precast concrete wall structures: less than 3/4 of the length of a perimeter wall. | | | |
| Quality of building materials is considered to be a dequate per the requirements of national codes and standards (an estimate). | | | |
| Quality of workmanship (based on visual inspection of few typical buildings) is considered to be good (per local construction standards). | | | |
| Buildings of this type are generally well maintained and there are no visible signs of deterioration of building | | | |
| | force effects from any horizontal direction that serves to transfer inertial forces from the building to the foundation. The building is regular with regards to both the plan and the elevation. The roof diaphragm is considered to be rigid and it is expected that the roof structure will maintain its integrity, i.e. shape and form, during an earthquake of intensity expected in this area. The floor diaphragm(s) are considered to be rigid and it is expected that the floor structure(s) will maintain its integrity during an earthquake of intensity expected in this area. There is no evidence of excessive foundation movement (e.g. settlement) that would affect the integrity or performance of the structure in an earthquake. The number of lines of w alls or frames in each principal direction is greater than or equal to 2. Height-to-thickness ratio of the shear walls at each floor level is: Less than 25 (concrete walls); Less than 13 (urreinforced masonry walls); Less than 13 (urreinforced masonry walls); Vertical load-bearing elements (columns, walls) are attached to the foundations; concrete columns and walls are doweled into the foundation. Exterior walls are anchored for out-of-plane seismic effects at each diaphragm level with metal anchors or straps The total width of door and window openings in a wall is: For brick masonry construction in cernent mortar : less than ½ of the distance betw een the adjacent cross walls; For adobe masonry, stone masonry and brick masonry in mud mortar: less than 1/3 of the distance betw een the adjacent cross walls; | Statement Yes The structure contains a complete load path for seismic force effects from any horizontal direction that serves to transfer inertial forces from the building to the foundation. Image: Content of Content on Content of Content on Content of Content on Content of Content on Content on Content of Content on Content of Content on Content of Content on Content of Content of Content of Content of Content on Content of Content on Content of Content on Cont | Note Yes No The structure contains a complete load path for seismic force effects from any horizontal direction that serves to transfer inertial forces from the building to the foundation. Image: Content of Co |

٦Г

٦

٦Г

5.2 Seismic Features

٦٢

Г

| Structural Element | | Earthquake Resilient Features | Earthquake Damage Patterns |
|---------------------------|--|----------------------------------|-------------------------------|
| Wall | Inadequate wood panel connections. | | |
| Frame (columns, beams) | | | |
| Roof and floors | Inadequate quality of roof-to-ceiling or roof-to-tie beam joints | | |
| Other | | | |

5.3 Overall Seismic Vulnerability Rating

The overall rating of the seismic vulnerability of the housing type is *C: MEDIUM VULNERABILITY (i.e., moderate seismic performance)*, the lower bound (i.e., the worst possible) is *C: MEDIUM VULNERABILITY (i.e., moderate seismic performance)*, and the upper bound (i.e., the best possible) is *D: MEDIUM-LOW VULNERABILITY (i.e., good seismic performance)*.

| Vulnerability | high | medium-high | medium | medium-low | low very low | |
|------------------------|-----------|-------------|----------|------------|--------------|-----------|
| | very poor | poor | moderate | good | very good | excellent |
| Vulnerability Class | А | В | С | D | Е | F |
| | | | | | | |

5.4 History of Past Earthquakes

Date Epicenter, region Magnitude Max. Intensity

Performance of this type of construction under destructive earthquakes has not been reported as yet.

6. Construction

6.1 Building Materials

| Structural element | Building material | Characteristic strength | Mix proportions/dimensions | Comments |
|--------------------------|-----------------------|------------------------------------|----------------------------|----------|
| Walls | Wooden (larch) panels | 500 - 800 kg/cm ² | | |
| Foundation | Concrete | 10 MPa (cube compressive strength) | | |
| Frames (beams & columns) | | | | |
| Roof and floor(s) | Wooden (larch) beam | 800 kg/cm ² | | |

6.2 Builder

Anyone can live in buildings of this construction type.

6.3 Construction Process, Problems and Phasing

Typically contractor builds construction of this type. Wood panels are fabricated in the workshop. For building assembly, in addition to the carpentry tools, auto-cranes and concrete mixers are also required. The construction of

this type of housing takes place in a single phase. Typically, the building is originally designed for its final constructed size.

6.4 Design and Construction Expertise

Expertise for design of buildings of this type was available, including the construction quality procedure developed by the author of this contribution. The design is carried out by Professional Engineers and Architects.

6.5 Building Codes and Standards

This construction type is addressed by the codes/standards of the country. Seria 181-115-77 cm according to the

Building Catalog of Typical Housing Projects, Vol.1, Part 2, Div.1, Seria 115, #15, 1984. The year the first

code/standard addressing this type of construction issued was 1984. SNiP II-7-81. Building in Seismic Regions-

Design Code. The most recent code/standard addressing this construction type issued was 1981. Title of the code or standard: Seria 181-115-77cm according to the Building Catalog of Typical Housing Projects, Vol.1, Part 2, Div.1, Seria 115, #15, 1984. Year the first code/standard addressing this type of construction issued: 1984 National building code, material codes and seismic codes/standards: SNiP II-7-81. Building in Seismic Regions-Design Code When was the most recent code/standard addressing this construction type issued? 1981.

The process consists of issuing permits for the design and construction, induding the architectural permits and urban planning/municipal permits. Designers need to have license to practice and are responsible to follow the building codes. Building inspection is performed and the permit is issued.

6.6 Building Permits and Development Control Rules

This type of construction is an engineered, and authorized as per development control rules. Building permits are required to build this housing type.

6.7 Building Maintenance

Typically, the building of this housing type is maintained by Owner(s). The maintenance is performed either by the owner (city) or (periodically) by a contractor - a maintenance firm.

6.8 Construction Economics

160 rub $/m^2$ (50-100 US/m^2)- per the official rate. 376 person-hours/building.

7. Insurance

Earthquake insurance for this construction type is typically available. For seismically strengthened existing buildings or new buildings incorporating seismically resilient features, an insurance premium discount or more complete coverage is unavailable. The insurance is available as a part of the usual property insurance. The Insurance covers about 3-5% of the total estimated property value.

8. Strengthening

8.1 Description of Seismic Strengthening Provisions

Strengthening of Existing Construction :

| Seismic Deficiency | Description of Seismic Strengthening provisions used |
|--------------------|--|
| Wood panels | Strengthening of joints |

8.2 Seismic Strengthening Adopted

Has seismic strengthening described in the above table been performed in design and construction practice, and if so, to what extent?

No. In general, seismic strengthening of this construction is not considered feasible.

Was the work done as a mitigation effort on an undamaged building, or as repair following an earthquake? N/A.

8.3 Construction and Performance of Seismic Strengthening

Was the construction inspected in the same manner as the new construction? N/A.

Who performed the construction seismic retrofit measures: a contractor, or owner/user? Was an architect or engineer involved?

N/A.

What was the performance of retrofitted buildings of this type in subsequent earthquakes? N/A.

Reference(s)

- 1. Manual on Certification of Buildings and Structures in the Seismic-Prone Areas, Second Edition CENDR, Petropavlovsk, Kamchatka, Russia 1990
- 2. Building Catalog of Typical Housing Projects, Vol.1, Part 2, Div.1, Seria 115, #15

Author(s)

- Mark Klyachko Director, Centre on EQE and NDR (CENDR)
 9 Pobeda Avenue, Petropavlovsk Kamchatka 683006, RUSSIA Email:cendr@svyaz.kamchatka.su cendr@peterlink.ru FAX: +7(415)22-8774 +7(812)222-0676
- Andrey Benin Senior Researcher, Centre on EQE and NDR (CENDR)
 9 Pobeda Ave., Petropavlovsk Kamchatka 683006, RUSSIA Email:cendr@svyaz.kamchatka.su or cendr@peterlink.ru FAX: (7-415) 22 8774; (7-812) 222 0
- 3. Janna Bogdanova Senior Researcher, Centre on EQE and NDR (CENDR)

9 Pobeda Avenue, Petropavlovsk Kamchatka 683006, RUSSIA Email:cendr@svyaz.kamchatka.su or cendr@peterlink.ru FAX: +7(415)22-8774 +7(812)222-0676

Reviewer(s)

 Svetlana Uranova Head of the Laboratory

 KRSU
 Bishkek 720000, KYRGYZSTAN
 Email:uransv@yahoo.com FAX: 996-3312-282859

Save page as

